

Hydraulics

3rd Year civil

First Term (2009 - 2010)

Chapter ()

2009 - 2010

1- Define: The permissible tractive force.

The critical tractive force.

Isovels.

2. State the factors that affect the velocity distribution in open channel.

Compare between hydraulically rough channel & hydraulically smooth channel.

4- The velocity distribution in a certain cross section show that the velocity equation at vertical section is given by:-

$$U = \frac{y_o^2}{4a} \left(1.4356 + 1.76 \frac{y}{y_o} - \left(\frac{y}{y_o} \right)^2 \right)$$

In which: $a = 0.0047yS^{(-2/3)}$

y measured from the river bed, and yo is the total depth.

Drive an expression for:-

a- Surface velocity.

b- Bottom velocity.

c- Mean velocity.

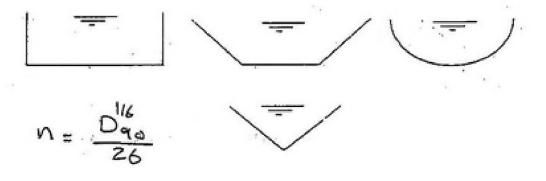
d- Maximum velocity.

5- Estimate the maximum shear stress on both the sides and the bottom of atapezoidal open channel if, b=4y=10.0 m, n=0.015, S=10.0 cm/km, Z=1.5, d₅₀=2.5mm, ysat.=1.8 Vm³, and the angle of repose=38°, show how to check the stability of the hydraulic section. Calculate the maximum tractive force ratio and the shear velocity.

6- If Q=42 m³/sec., Z=2, S=12 cm/km , d₅₀=4mm , Φ=30° , d₅₀=7mm , γsat=2.65, Design the canal section using the critical shear stress method.

7- In a river of bed width of 600 m and bed slope of 7.50 cm/km. It is found that the bed material just begin to move when the discharge is 420 million m3/day. Assuming the mean velocity to vary with the water depth and slope according to the relation, V=120ys^{3/3}, find the bed slope at which the same tractive force on the bed would be produced with a discharge of 365 million m³/day.

8- Plot Isovels and shear stress distribution for the following sections.



$$Q_{(4)}: U = \frac{y_0^3}{49} \left[1.4356 + 1.76 \frac{y}{y_0} - \left(\frac{y}{y_0} \right)^2 \right]$$

J: measured from river bed .

Jo: total depth.

a = 0.00 47 % .5 - 2/3

Req : Orive an expression for

a - Surface velocity.

b - bottom velocity.

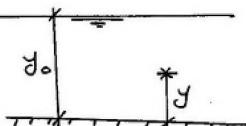
C - Max. velocity.

d- mean velocity.

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For surface velocity.

d = 80



.. Usurface = $\frac{J_o^2}{49} \left[1.4356 + 1.76 \left(\frac{J_o}{J_o} \right) - \left(\frac{J_o}{J_o} \right)^2 \right]$

Usurface = 49 [1.4356+1.76-1]

Usurface = 0.5489 4

For bottom velocity.

g = 0

Ubottom = 40 [1.4356 + 1.76 (4) - (0)]

UboHom = 0.3589 4

For max velocity:

$$\frac{du}{dy} = 0$$

$$0 = \frac{y^{2}}{4a} \left[0 + 1.76 \left(\frac{1}{y_{0}} \right) - \frac{2y}{y_{0}^{2}} \right]$$

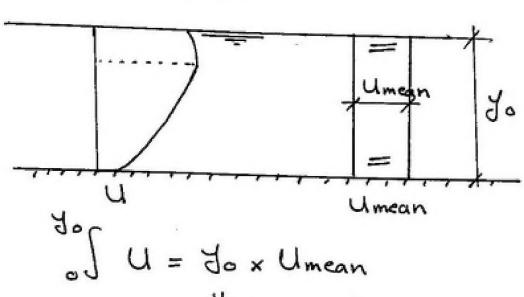
$$\frac{1.76}{y_{0}} = \frac{2y}{y_{0}^{2}}$$

$$\therefore 1.76 y_{0}^{2} = 2 y_{0}^{2}$$

$$1.76 y_{0} = 2y$$

$$y = 0.88 y_{0}$$

For mean velocity:



Umean =
$$\frac{1}{30} \int_{40}^{30} \left(\frac{1.4356}{40} + \frac{1.769}{90} - \frac{y^2}{90^2} \right) dy$$

Umean = $\frac{1}{40} \int_{40}^{30} \left(\frac{1.4356}{1.4356} + \frac{1.769}{90} - \frac{y^2}{90^2} \right) dy$
= $\frac{1}{40} \left[\frac{1.4356}{290} + \frac{1.769}{290} - \frac{y^3}{390^2} \right]_{0}^{30}$

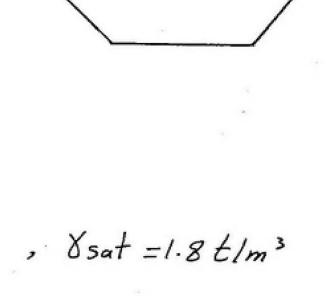
Umean =
$$\frac{y_0}{4a} \left[1.4356 \, y_0 + 0.88 \, y_0 - \frac{y_0}{3} \right]$$

Umean = $\frac{y_0^2}{4a} \left[1.4356 + 0.88 - 0.33 \right]$

Umean = $0.4964 \, \frac{y_0^2}{a} \, \#$

Q(5):

Given :



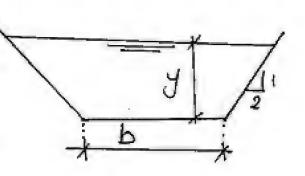
Req.: 2 - check stability. 3 - Tractive force ratio (K) 4 - Shear Velocity (Ux) 50/. : (1) : b = 44 => b = 10m 7 = 2.5m " b=4y , Z=1.5 " Zs' = 0.75 8.4.5' = 0.75 x 1000 x 2.5 x (10x10-5) Ts = 0.19 Kg/m2 # " Zb = 0.97 8.4.5 =0.97 x 1000 x 2.5x (10 x10-5)

Q(6):

Reg: - Design section using critical shear stress method.

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assume)



for stability Ts + Zer . 0.4 = 0.76 x 1000 x y x (12x10-5) J = 4.40 m For stability Tb + Ter 0.4 = 0.98x 1000 x 4x (12x10-5) y = 3.40 m لفمام ليتزان نأخذ (٤) الدُقار. : b=4x3.4 = 13.60m ·· Q = 1 AS13 . 5/2 A = (13.6+ 2x3.4) x 3.4 = A = 69.36 m2 P = 13.6 + 2x 3.4 /1+22 P = 28.80 m

$$n = \frac{(0.007)^{1/6}}{26} = 0.0168$$

(safe)

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Q(7) :

Given:
$$-B = 600 \text{ m}$$
, $S = 7.5 \text{ cm/km}$
 $-Q = 120 \text{ million } m^3 / day$
 $-V = 120 \cdot y \cdot 5^{12/3}$

Req. :

501 .:

$$Q = \frac{120 \times 10^6}{24 \times 60 \times 60} = 1388.9 \text{ m}^3 \text{ls}$$

$$A = 600 \text{ y}$$

$$b = 600$$

1388.9 =
$$(600 \text{ y}) \times 120 \text{ y} \times (7.5 \times 16^{-5})^{\frac{3}{3}}$$
 $\exists = 3.30 \text{ m}$
 $\therefore \quad \mathcal{Z} = 8. \text{ y}. \text{ s}$
 $\therefore \quad \mathcal{Z} = 1000 \times 3.30 \times (7.5 \times 10^{-5})$
 $\mathcal{Z} = 0.25 \text{ kg/m}^2$

For $Q = 365 \times 10^6 \text{ m}^3 \text{ d}$
 $\therefore \quad \text{V} = 120 \cdot \text{y} \cdot \text{s}^{\frac{2}{3}}$
 $\text{Told} = \text{Tnew}$
 $0.25 = 1000 \times \text{y} \times \text{s} \longrightarrow \text{II}$
 $\therefore \quad Q = A \times V$
 $A = 600 \text{ y}$
 $\therefore \quad Q = \frac{365 \times 10^6}{94 \times 60 \times 60} = 4224.5 \text{ m}^3 \text{ s}$

$$0.059 = 3^{2} \cdot 5^{2} \cdot 5^{2$$